Memo

TO: Josie Tayse, Final Design Project Manager

Metro District, Waters Edge

FROM: Paul Martin, Assistant Foundations Engineer

Geotechnical Engineering Section

Concur: Rich Lamb, Foundations Design Build Engineer

Geotechnical Engineering Section

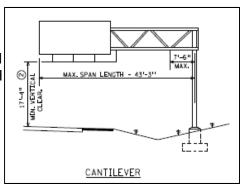
DATE: December 19, 2017

SUBJECT: SP 8825-652, Metrowide Overhead Sign Structure Replacement

Subsurface Evaluation and Foundation Recommendation REVISED Memo

1.0 Project Summary

This REVISED report is provided in response to a request by the Metro District to provide a subsurface investigation and foundation recommendations for eight proposed new overhead sign structures being constructed along TH 7, TH 36, TH 47, TH 94 (two sites), TH 100 (two sites) and TH 280. The revisions include the removal of X-section sheets showing inaccurate elevations and the inclusion of subsurface information and recommendations from a previous project (SP 8825-562) for two locations on TH 35 that have been rolled into this project. The sign structure types will include both Sign Bridge and Cantilever.



2.0 Subsurface Investigation

With the exception of Site 13, the soils at the proposed sign locations were investigated using Standard Penetration Test Borings (SPT) or Cone Penetration Test Soundings (CPT) conducted in May, June, October, November and December, 2017. Site 13 on TH 280 was explored in 1969.

The CPT soundings were interpreted for general soil behavior type and estimated water table elevation. No soil samples were taken so the interpreted soil behavior type may not exactly match what soil is present, but should indicate how it behaves if compared to standard soils.

Based on the results of the investigations we determined that the foundation soils at the proposed sign locations include loose to very dense sands and layered soft to stiff sandy loam, clay and silt soils. Groundwater was detected in some of the borings and CPT soundings and perched water should also be expected in areas where layered soils exist. Please refer to the attached sounding and boring logs for the most complete description of the foundation soils.

Table 1, below, provides general summaries of the soil and groundwater conditions encountered at/near each sign location.

1



















Table 1, Summary of Estimated Soil and Groundwater Conditions

Boring or	Location	Indicated Groundwater	Summary of Interpreted Soils	Interpreted
Sounding		Depth (feet)	(Depths in feet) Sandy Loam and Sand to 9'	N-Value 25-41
T01	Site 1, TH 7 WB		Clays and Silts to 16.5'	25-41 12-19
101	STA 311+26, 36' Lt	6.9	Sandy Loam and Clay Loam w/Sand & Gravel	27-39
T05	Site 3, I35 NB STA 1683	0.5	Sandy Louin and Clay Louin Wy Sand & Graver	27 33
(562)	near Greenhaven Dr	NI-36'	Mostly medium dense to loose Sands	2-15
C07 (562)	Site 5, 135 NB STA 1717+15, near Southcross Dr.	*	Mostly Sands and Sandy Loam with a few Silt and Clay layers	3-50+
	Southeross Dr.		Frozen soils and Sands to 7'	20-50+
C02	Site 7, TH 36 EB Ramp to		Layered Silts, Clays and Sands to about 25'	3-20
002	SB TH 694	*6.5	Sandy Loam and Sands to termination	14-36
T03	Site 8, TH 47 SB, STA 523+79, 43' Rt	8.5	Fine and very fine grained sands	12-85
T04	Site 8, TH 47 SB,			
104	STA 523+81, 20' Lt	11.2	Fine sand, very fine sand and loamy sand	3-66
T05	Site 9, TH 94 WB	•	Plastic Silt Loam to 10'	15-33
	STA 2118+46, 31 Lt	NI-36'	Sands & Gravel to termination.	21-48
TOC	C:+- 40 TH 04 M/D		Loamy sand to 4, clay loam to 6.5,	23,12
T06	Site 10, TH 94 WB,	*	silt loam to 9, sands to 19,	8,7-23
	STA 428+52, 52' Lt	•	sandy clay to 31.5, sandy loam Sand fill to 11.5, topsoil to 13,	10-13,9-17 18-41, 14
T07	Site 10, TH 94 WB,		sand to 21.5, clay to 31.5, over	8-21,6-11
107	STA 428+55, 28' Rt	*	sandy loam and silty clay	6-21,0-11
	Site 11, TH 100 SB		Layered Clays, Sands and Silts to 10.5'	3-32
C08a	STA 183+01, 42'Rt	*	Refusal on dense Sand	50+
600	Site 12, TH 100 SB		Sands layered with Silt and Clay to 29.5'	4-50
C09	STA 227+10, 32'Lt	*	Refusal on Dense Sand	30-50+
TC-2	Site 13, TH 280 NB near		Plastic Sandy Loam to 13'	11-16
(1969)	STA 88+60	25	Sand & Gravel to termination.	20-75

*Perched layers may occur

NI- Not Indicated to

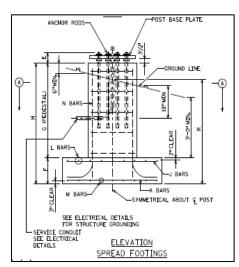
3.0 Foundation analysis

As part of the overhead sign standard drawings (revised 11-21-2014), standard foundations were developed to support overhead signs. These standard foundations consist of two spread footing and two drilled shaft designs to be used on the various sign post sizes (see Drawing ST-3, Standard Overhead Sign Supports Interim Design B).

- Spread Footings: 12 ½ ft. x 18 ft. or 9 ft. x 14 ft.
- Drilled Shafts: 3 ft. diameter, 23 ft. deep or 4 ft. diameter, 29 ft. deep

In addition, the standard foundation notes state the following requirements:

 All spread footings shall have an allowable design bearing pressure of 1 ¼ tons per square foot



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2









 The drilled shafts have an allowable design lateral bearing pressure of 250 lbs. per square foot per foot of depth

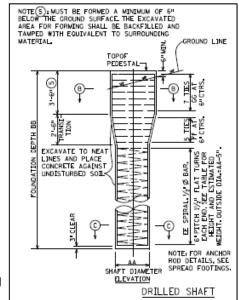
Based on previous research, these requirements will be met in all but the weakest mineral soils, and therefore represent a very conservative design. In lieu of new foundation designs we checked the foundation soils to see if they met those requirements.

The recommendations contained in the paragraphs below are based on our analysis. We recommend the new foundations consist of drilled shafts designed to meet the requirements of the current standard design tables. At many locations the soils in the shaft excavations will require support to prevent caving to prevent loss-of-ground below nearby pavements, utilities or structures.

4.0 Foundation Recommendations

Based on review of the existing subsurface conditions and proposed structures, we recommend:

- 1. The overhead sign structures be supported with drilled shaft foundations as detailed in the typical Foundation Detail Sheet, Drawing ST-3 located on the Traffic Engineering website.
- Table 1 and the attached Overhead Sign Foundation Recommendations Table, showing the estimated soil and water conditions be forwarded along with a copy of the attached Boring and Sounding Logs to the bidding contractors.



- 3. That caving conditions be expected where excavations must extend below groundwater and perhaps other areas having very loose sands and gravel layers.
- 4. And that caving be controlled to protect existing pavements, utilities and structures.

Attachments:

Soil Boring Location Sketches, Sites 1, 3, 5, 7, 8, 9, 10, 11, 12, 13

Overhead Sign Foundation Recommendations Table

Boring Logs T01, T05 (562), T03, T04, T05, T06, T07, TC-2 (1969) Unique Numbers 82770, 82035, 82713, 82714, 82769, 82715, 82716

Sounding Logs C02, C07 (562), C08, C08a, C09, Unique Numbers 82817, 82536, 82796, 82824, 82818

SPT Index

CPT Index

cc: J. Tayse, Signing Engineer

E. Peterson, Metro Signing Design

E. Embacher, Construction Engineer

M. Waters, Environmental Stewardship

B. Skow, Chief Geotechnical Engineer

3

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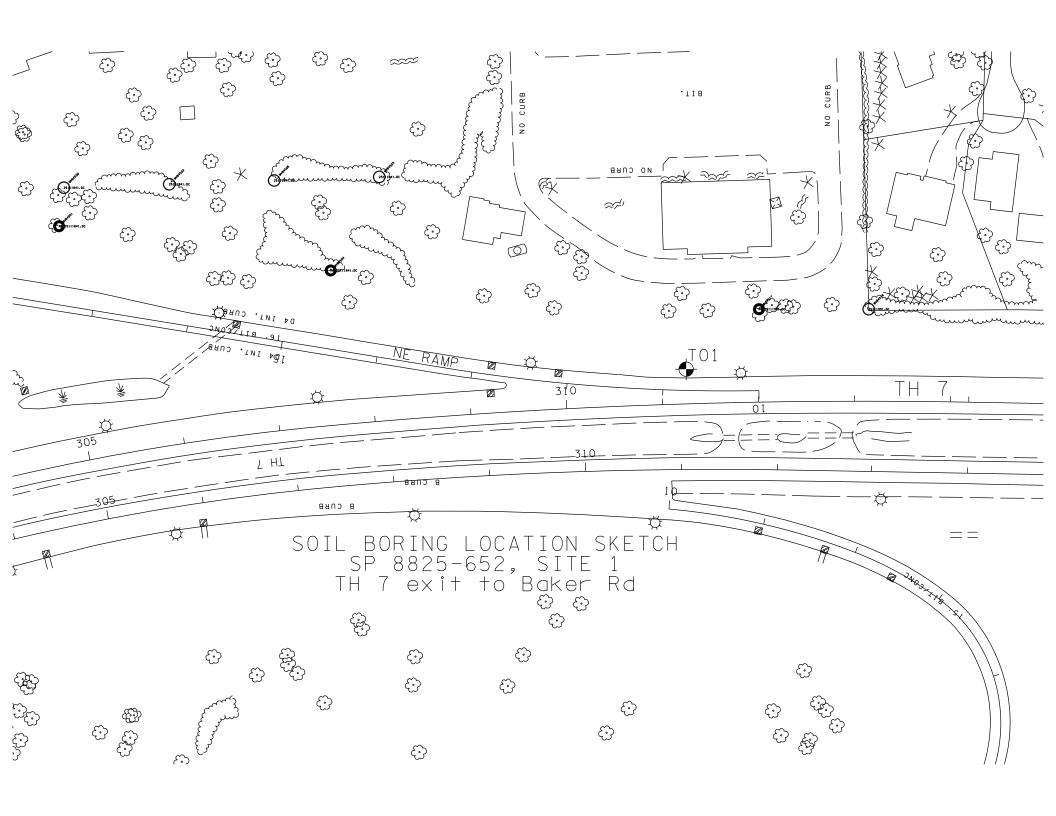


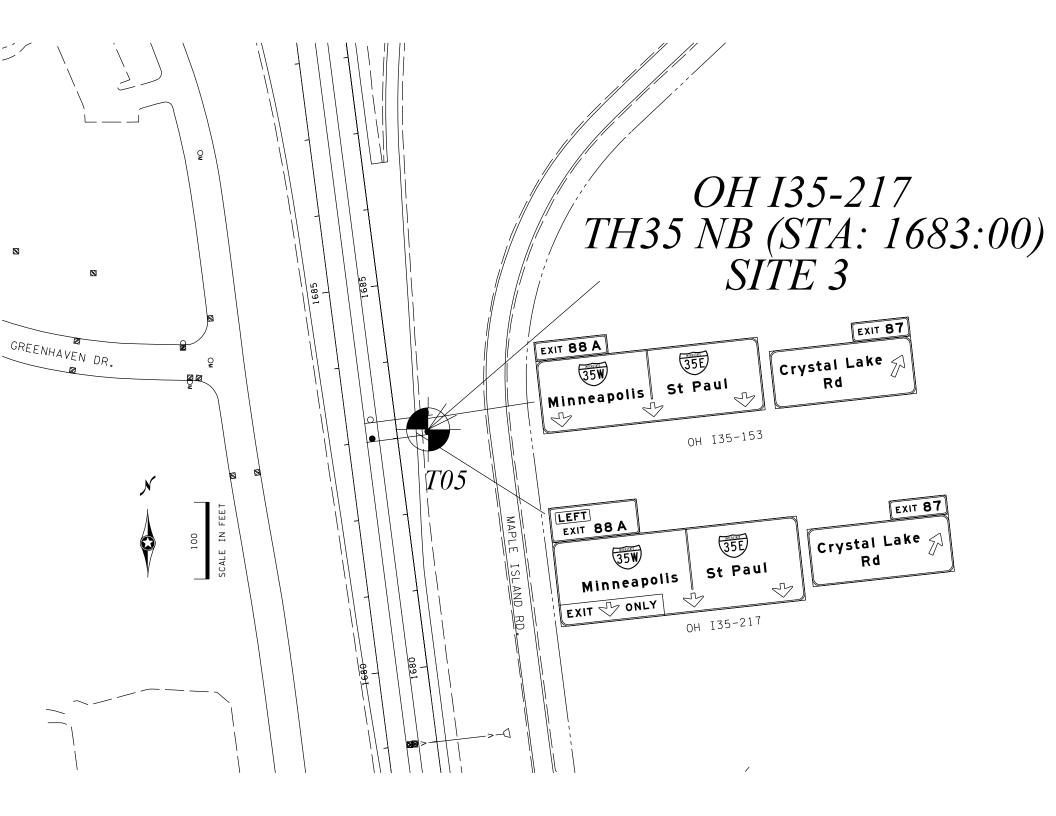


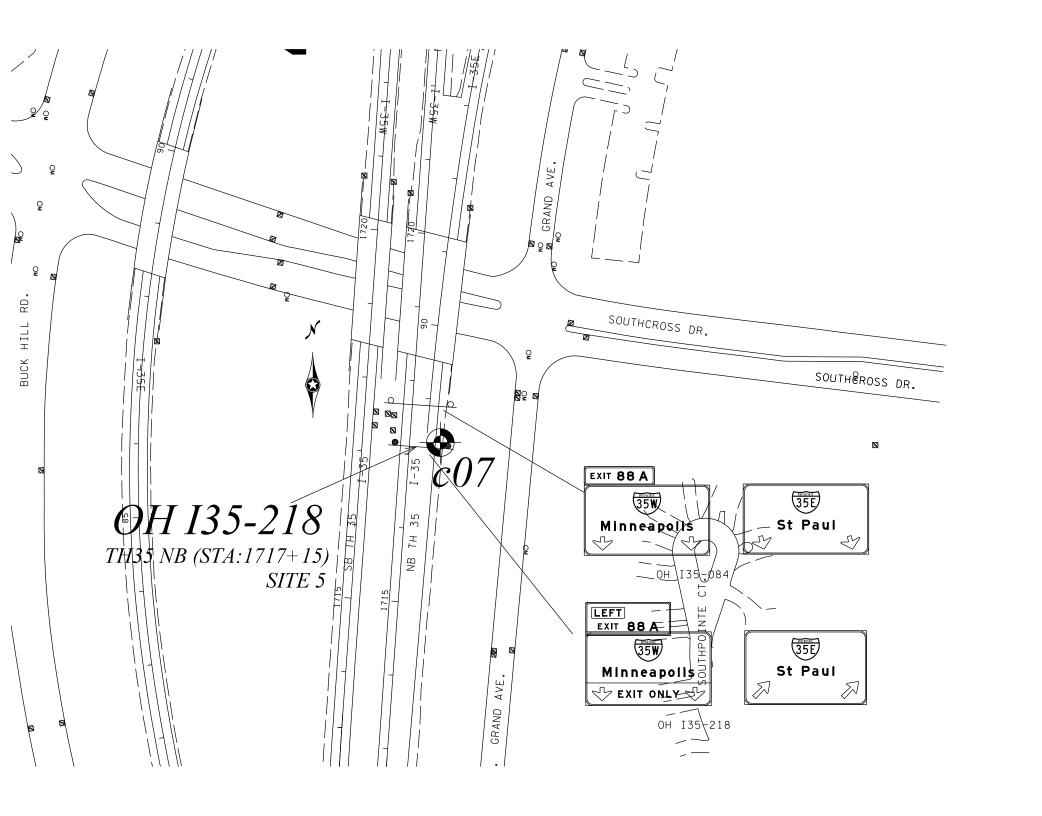


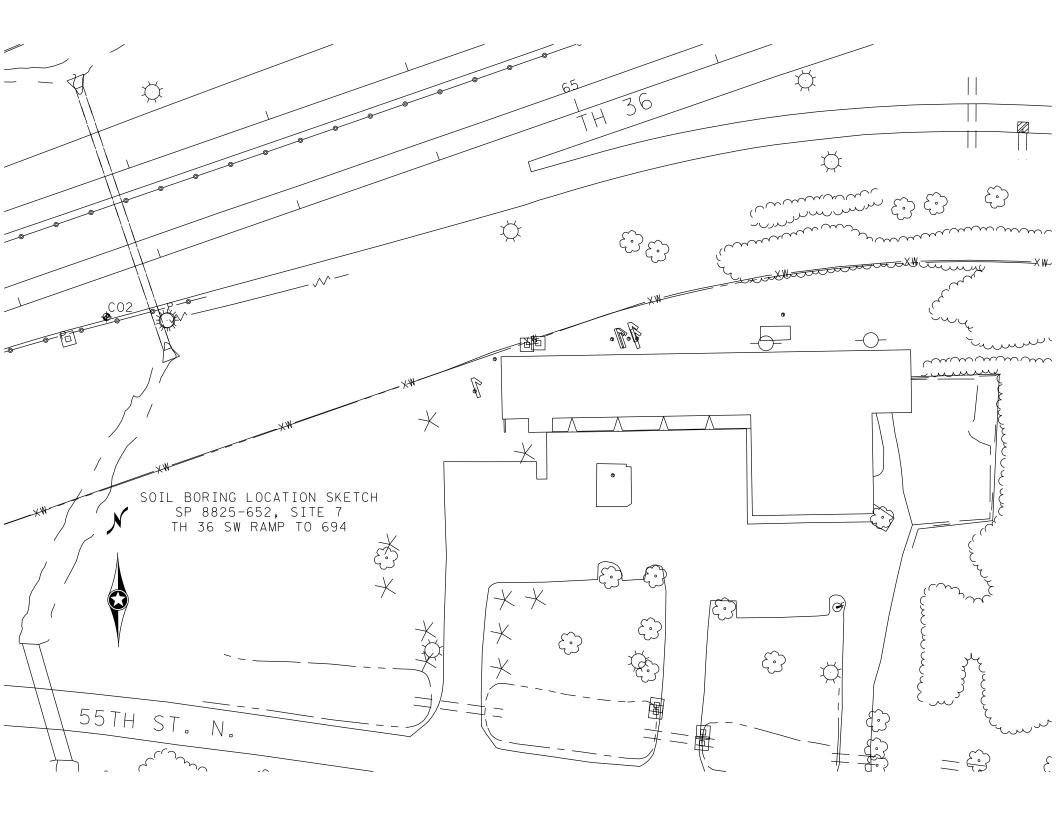


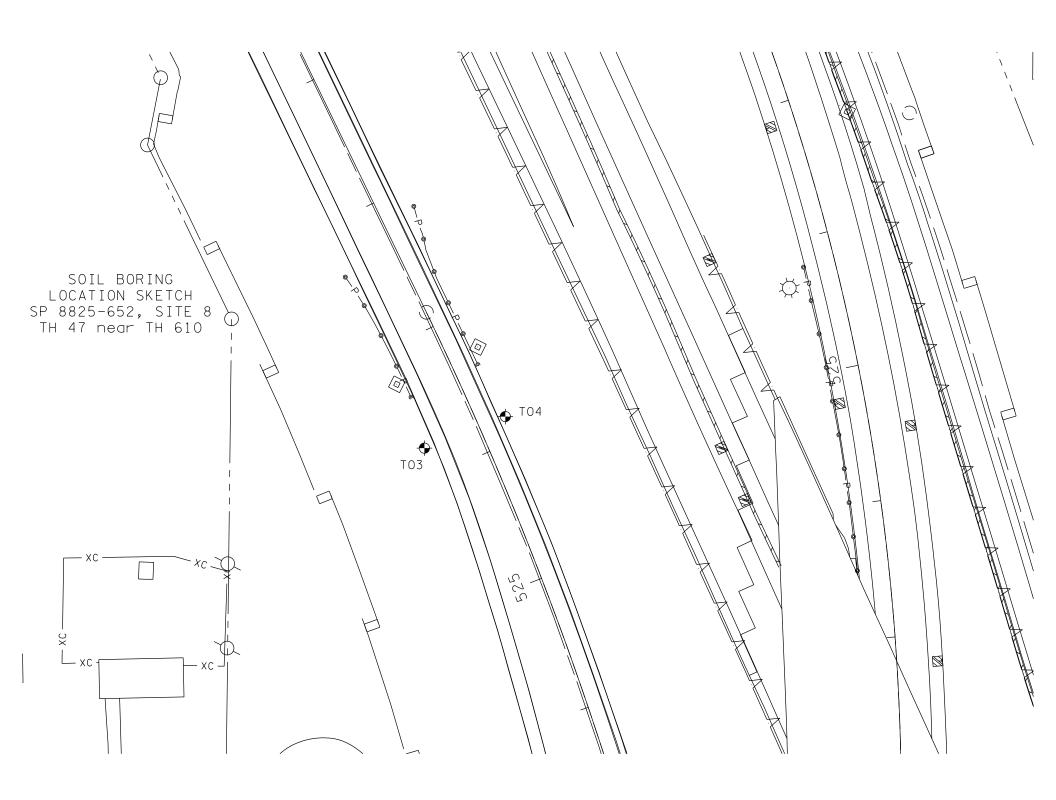


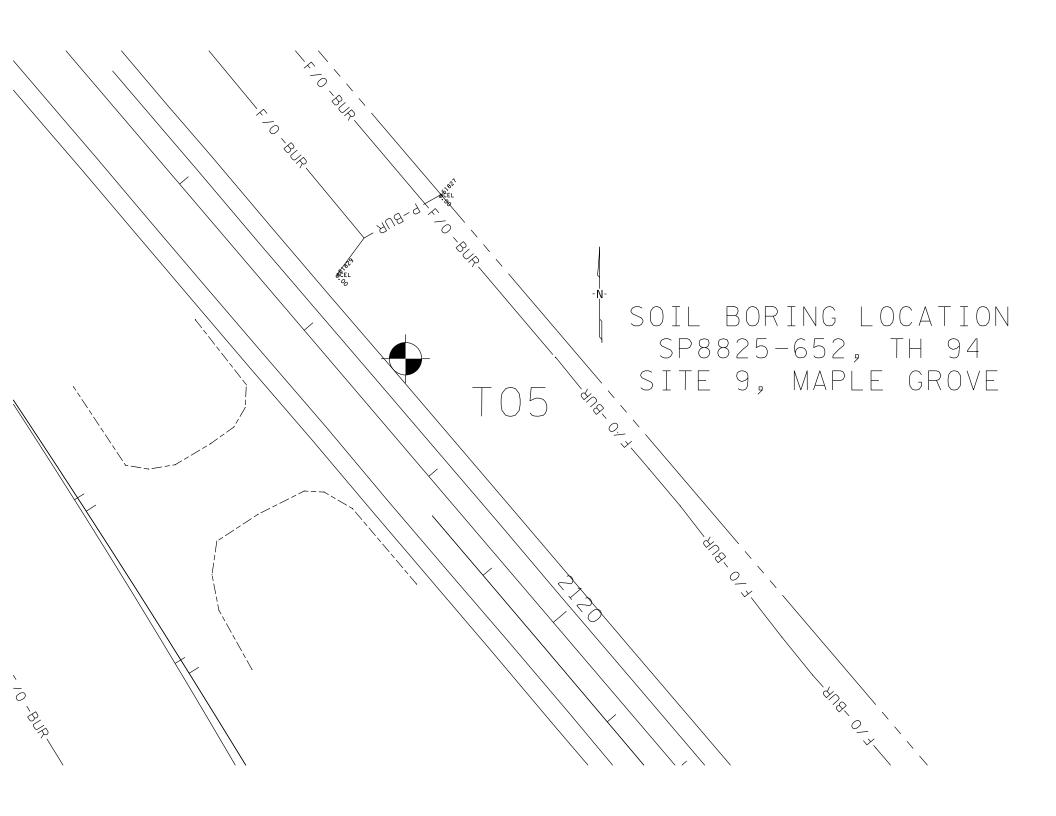


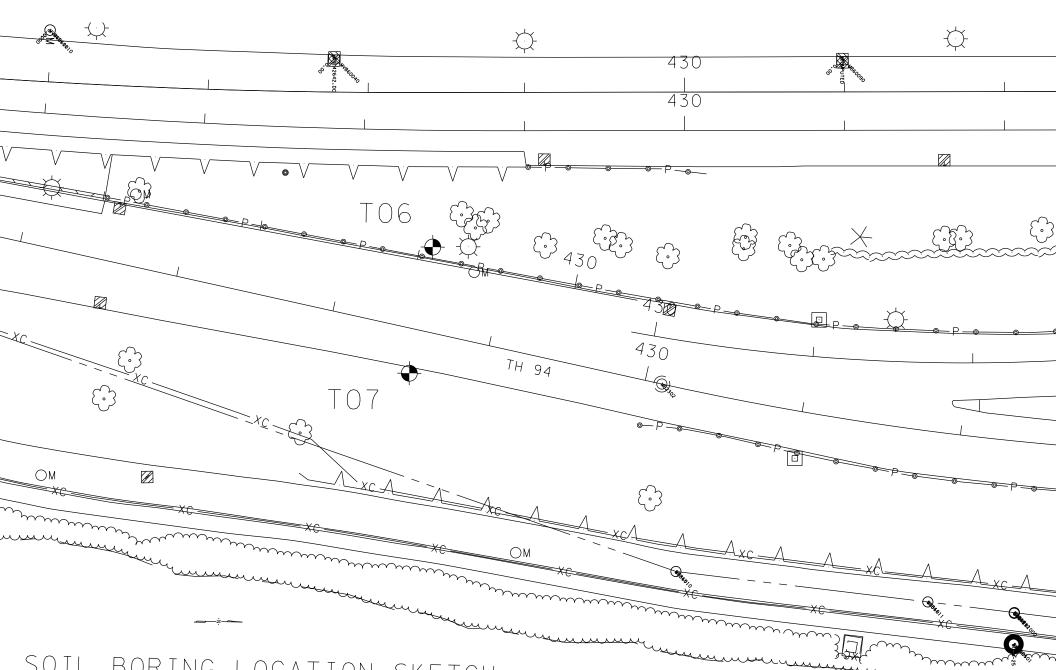




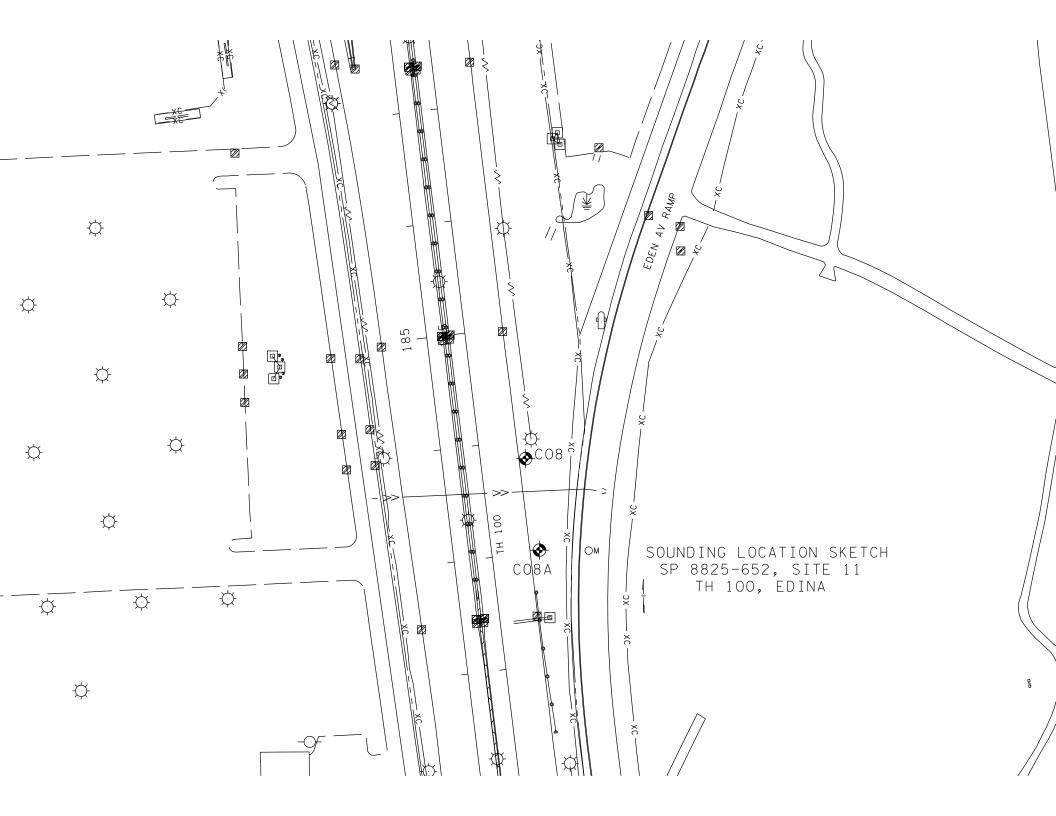


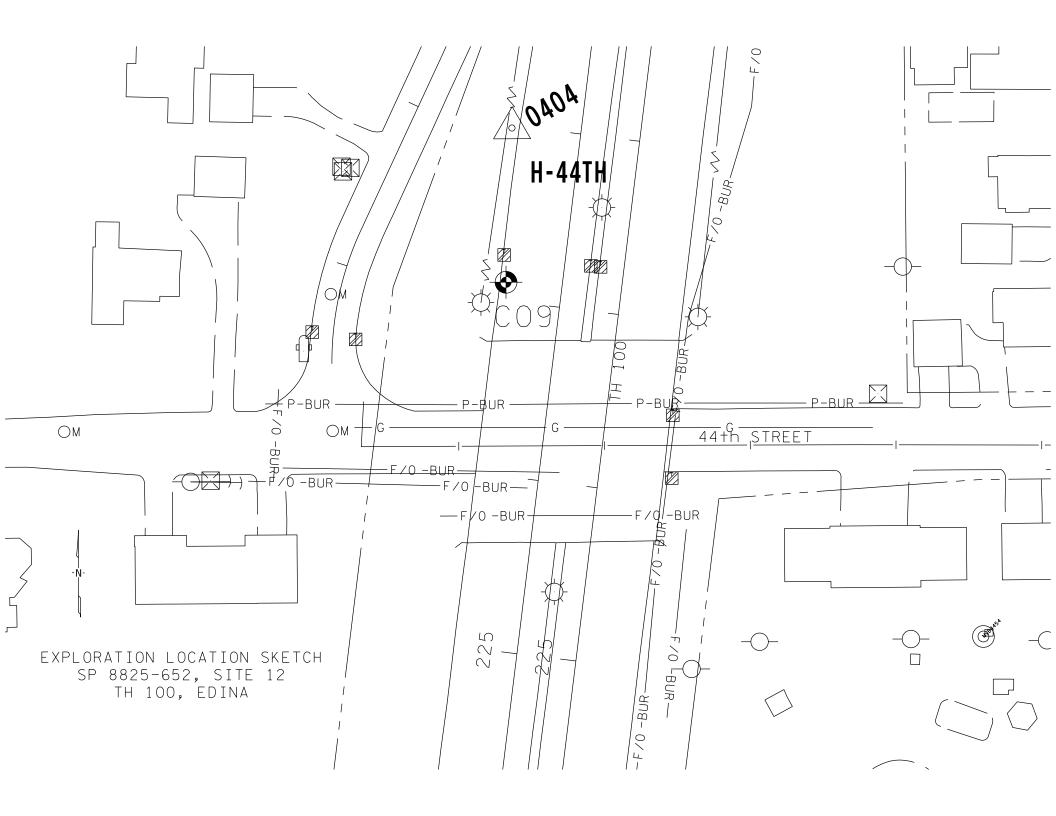


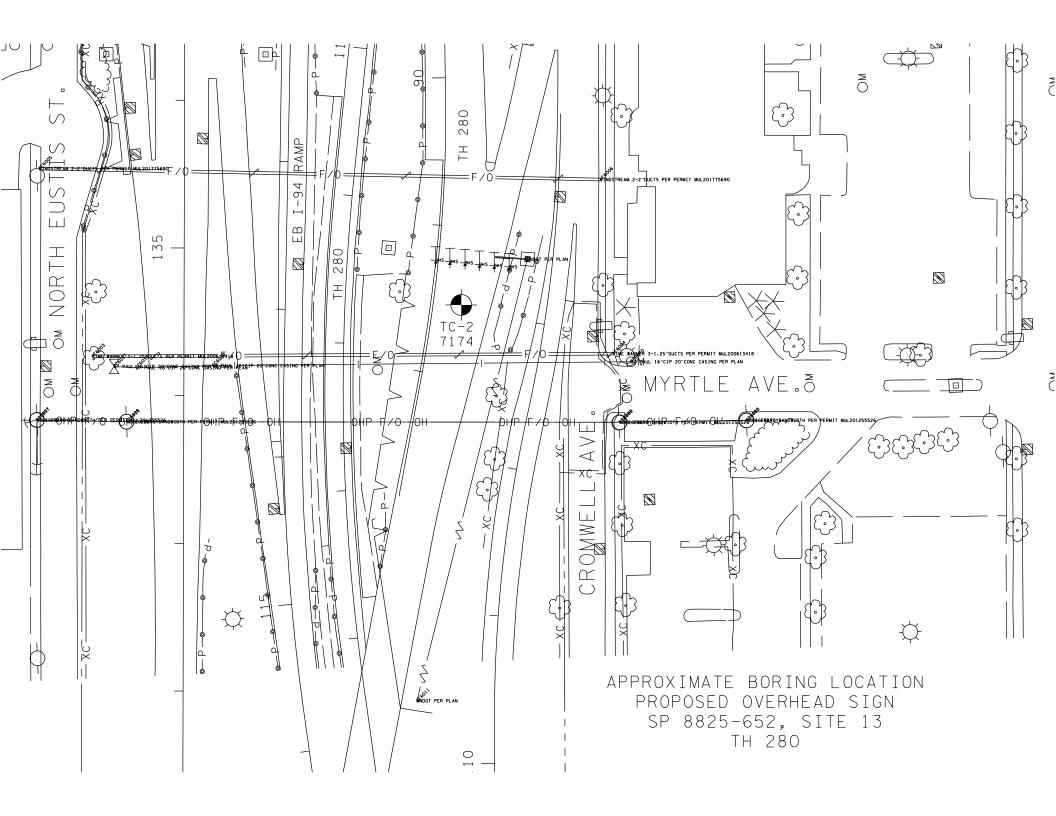




SOIL BORING LOCATION SKETCH SP 8825-652 Site 10, I-94 Borings TO6 and TO7







				SP 8825-6	552, Various N	Лetro State Highw	ays, Overhea	d Sign Foundation Recommendation	s	
Current Plan		Roadway	Direction	New Structure	Exploration	Exploration	Post	Soils	Ground	Recommendations
	ID	Type		Station	Points	Locations	Locations		Water	
NEW CANTILEVER , SITE 1, TH 7	MN7-001	RAMP	WB	Ramp to NB US Baker Road, STA 311+25, 110' west of existing sign	T01	311+26, 36'Lt	SHOULDER	Sandy Loam and Sand to 9', Clays and Silts to 16.5', then Sandy Loam and Clay Loam with layers of Sand & Gravel.	6.9'	Suitable for Standard Design, but excavations extending below the groundwater surface will likely require support, such as casing and/or drilling fluid.
NEW SIGN BRIDGE SITE 3, TH35	OH I35217	MAINLINE	NB	1683+00 NEAR GREENHAVEN DR.	T05, SP8825- 562	1683+06, 41' Rt	SHOULDER MEDIAN	Sands and Sandy Loam with Clay and Silt layers.	Below 979	Suitable for Standard Design
NEW SIGN BRIDGE SITE 5, TH 35	OH 135-218	MAINLINE	NB	1717+15 NEAR SOUTHCROSS DR.	C07, SP8825- 562	1717+19, 41' Rt	SHOULDER MEDIAN	Sands with layers of Sandy Loam, Clay and Silt.	*	Suitable for Standard Design, but excavations extending below the groundwater surface will likely require support, such as casing and/or drilling fluid.
NEW CANTILEVER, SITE 7, TH 36	MN36-090	RAMP	EB	Ramp to SB TH 694, STA 61+45, 25' east of existing sign	C02	61+52, 26' Rt	SHOULDER	Frozen soils and Sands to 7', layered Silts, Clays and Sands to about 25', Sandy Loam and Sands to 36'	*6.5	Suitable for Standard Design, but excavations extending below the groundwater surface will likely require support, such as casing and/or drilling fluid.
NEW SIGN BRIDGE, SITE 8 TH 47	MN47-004	MAINLINE	SB	TH 47 STA 523+75, 50' south of existing sign		523+79, 43'Rt 523+81, 20'Lt	SHOULDER MEDIAN	Fine and very fine Sands	8.5 11.2	Suitable for Standard Design, but excavations extending below the groundwater surface will likely require support, such as casing and/or drilling fluid.
NEW CANTILEVER, SITE 9, TH 94	194-688	MAINLINE	WB	2118+30, 30' southeast of existing sign	T05	2118+46, 31' Lt	SHOULDER	Silt Loam to 10', Sandy Loam to 11.5' and Sand & Gravel to 36'		Suitable for Standard Design
NEW SIGN BRIDGE, SITE 10, TH 94	194-689	MAINLINE	WB	428+50, 250' south of existing sign	T06 T07	428+52, 52'Lt 428+55, 28' Rt	MEDIAN SHOULDER	Sands with Clay and Silt layers.	Below 810	Suitable for Standard Design
NEW CANTILEVER, SITE 11, TH 100	MN100-142	MAINLINE	NB	182+80, 30' north of existing sign	C08a C08	183+01, 42'Rt 183+83, 40'Rt	SHOULDER	Layered Clays, Sands and Silts to 10.5'. Refusal on dense Sand	*	Suitable for Standard Design, but excavations extending below the groundwater surface will likely require support, such as casing and/or drilling fluid.
NEW CANTILEVER, SITE 12, TH 100	MN100-143	MAINLINE	SB	227+13, 25' South of existing sign bridge	C09	227+10, 32'Lt	SHOULDER	Sands with Clay and Silt layers to 29.5'. Refusal on dense Sand	*	Suitable for Standard Design, but excavations extending below the groundwater surface will likely require support, such as casing and/or drilling fluid.
NEW CANTILEVER, SITE 13, TH 280	MN280-023	MAINLINE	NB	88+60, 25' south of existing sign	TC-2 (1969)	NEAR 88+60	SHOULDER	Sandy Loam to 13', then Sand & Gravel to 34'	25	Suitable for Standard Design, but excavations extending below the groundwater surface will likely require support, such as casing and/or drilling fluid.
		•			•			Groundwater not indicated	* Perched gro sidewall supp	undwater layers may occur and may require excavation ort.



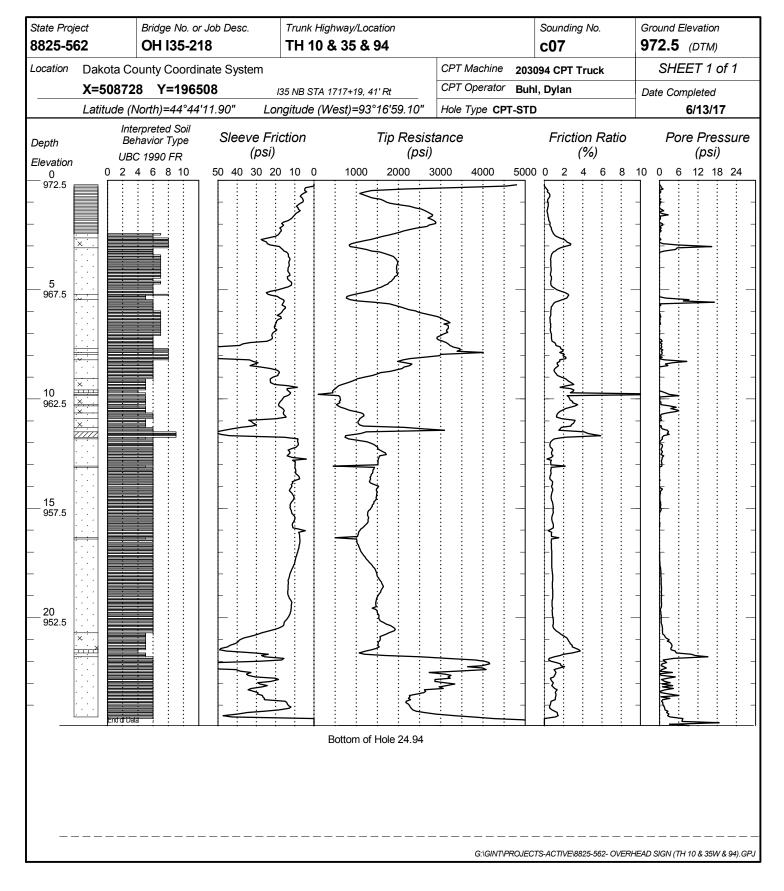
	Bridge No. or Job Desc. OH SIGN MN7-001 Trunk Highway/Location Various Metro							Boring I	Vo.		Ground Elevation 1004.4(DTM)	
	Landing					Drill Machine 211304 CME Fa					SHEET 1 of 1	
Locatio	Raili		County Coordinate System								Duilling	
				STA 311+26, 36' Lt	Han	Hammer CME Automatic Calibrated Compl					Completed 11/15/17	
	Latitude (North)=44°55'35.22" Longitude (West)=93°26'37.79" □ Depth ≳						MC (%)	COH (psf)	γ (pcf)	Soil	Other Tests Or Remarks	
DЕРТН	Берш	Lithology			ng ation	N60 REC	RQD				:	
Эa	Elev.	Lift	Clas	Drilling Operation	(%)	(%)	(ft)	Core Breaks	Roc	Formation or Member		
-		× · . · . · . × · . · . · . × · . · .	slightly plastic Sandy Loam with	<u> </u>	25	- 11						
5-	- - 6.5	· · · · × · · · · · · · · · · · · · · ·	dense to dense		<u> </u>	41	8			a	ittle rough drilling 3.5', 7'	
▼	997.9		Loamy Sand, gray-brown, satur	rated, medium dense	1	25 .	15					
10	995.4		Clay, brown, wet, stiff		PD	13	42					
+	_ 15.0		Clay, blown, wet, still	PD	12 .	34						
15	989.4 16.5		Sandy Clay Loam, browns with	X	19	19			ro	ugh drilling at 15.8'		
+	987.9 17.5 986.9	, ´o,	Silty Clay Loam, browns with IC Loamy Sand and Gravel, brow dense		PD	25 .	33 10				3.7.	
20	19.0 - 985.4 - 21.5		Sandy Clay Loam with pebbles	, brown, wet, very stiff	PD	27	20					
+	982.9	× · . · . · . × · .			PD	39 .	17			ro	ugh drilling at 21.5'	
25	-	× · . · . · . × · .			PD	37	11					
1	-	× · . × · . × · .	slightly plastic Sandy Loam with Loamy Sand and Gravel at 23.		PD	34	14					
30	<u>-</u> -	×			PD	35	13					
75	-	× · .			PD	34 .	10					
35	36.0 968.4	[, . ; ×	Bottom of Hole - 36.0'		X	31	11					
	500.4		Water measured at 6.9' with au	uger								
-				Field Crew	Chief						class: Edit: Date: 12/12/17 DISTRICT-OVERHEAD SIGNS.GPJ	



State Project Bridge No. or Job Desc. Trunk Highway/Location 8825-562 OH I35-217 Interstate Highway				H 10 8	& 35 8	k 94	Boring I	Vo.		Ground Elevation 1011.0 (DTM)		
ocation D	akc	ta Co	ounty Coordinate System		Drill	Machin	e 20933	2 GeoPr	obe		SHEET 1 of 1	
			1/ /	1683+06, 41' Rt	Hammer GeoProbe Automatic Drilling Completed						5/17/17	
L	.atitu	ıde (İ		ude (West)=93°16'55.72"		SPT	МС	сон	γ	1	Othora	ests
_т ∣ Dep	oth	gy			5	N 60	(%)	(psf)	(pcf)	Soil	Or Rem	
HEAD Dep		Lithology			Drilling Operation	REC	RQD	ACL	Core	ક	Forma	tion
				sification	D D	(%)	(%)	(ft)	Core Breaks	å	or Men	
0.5 - 1010		Z1 12.	Topsoil with roots, dark-brown, I	moist	$\dashv \times$	15	33					
+												
+			ledium-Grained Sand, brown, damp, medium dense to loose			14	3					
+			Medium-Grained Sand, brown,	damp, medium dense to loose	 		+					
5+						8	4					
7.0												
1004	1.0			13	6	4						
+						+ *						
10+												
+					\downarrow	10	9					
†												
Ī					X	2	8					
15 +	Fine-Grained Sand, brown, damp, loose to very loose			13		1						
+					X	12	9					
+					3		+					
†					\times	14	7					
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22.0					17		+					01 07 01
989.	.0					12	- 5			ro	ough drilling 22.	.0'-27.0'
+					[]		+					
25 +						7	4					
Ī			Medium-Grained Sand with son	ne Gravel, brown, damp, loose	1	'	Ī •					
1			to medium dense		17		ļ ,					
+						9	3					
30 +		: - :			1		+ _					
31.					\perp	11	_ 8					
979.	.Э		Bottom of Hole -31.5' No water encountered or measurements.	red during drilling								
				5 5								
				-	 rew Ch	 ief: Buhi	 I, Dylan S	 Soil Class	 S:JAH Roo	 ck C	 Class: Edit: Da	ate: 12/18/
						G:\GIN	IT/PROJEC	TS-ACTIVE	8825-562- O	VERI	HEAD SIGN (TH 10	& 35W & 94).

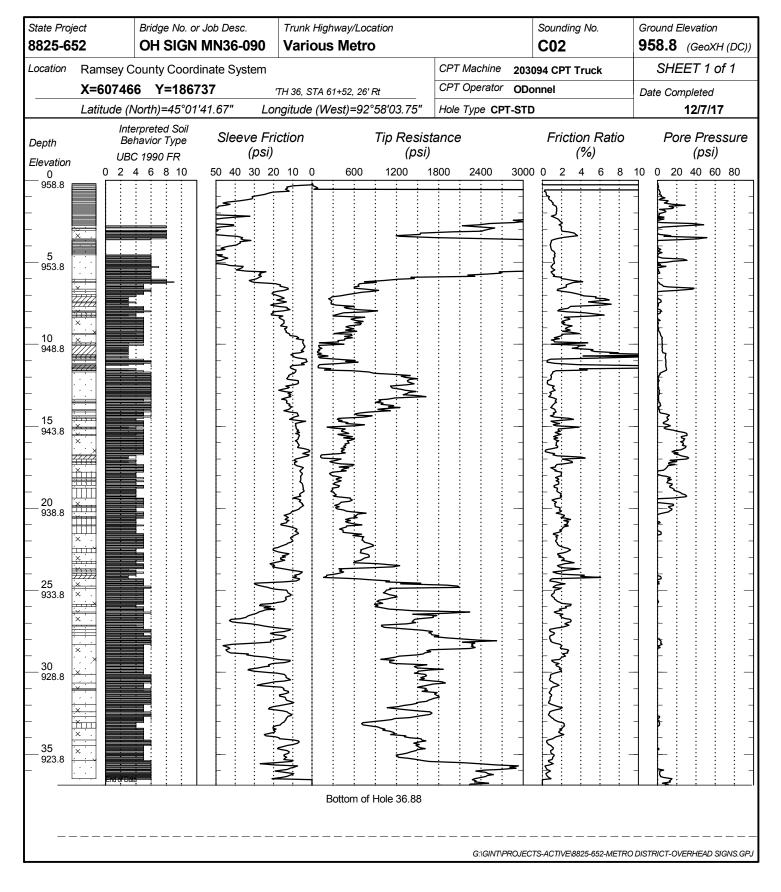


CONE PENETRATION TEST RESULTS





CONE PENETRATION TEST RESULTS





	Project -652		Bridge No. or Job Desc. OH SIGN MN47-004 SB Various Metro				Boring I			Ground Elevation 894.3 (DTM)		
ocatio	on Ram	isey C	County Coordinate System	Drill	Machine	e 21130	4 CME F	at Tire	SHEET 1 of 1			
	X=	5289 ⁻	15 Y=229792 _{TH 47 SB, STA 523+79, 43'Rt}	Han	nmer CN	/IE Auto	matic Ca	alibrated		Drilling 10/24/17 Completed		
	Latitude (North)=45°08'49.28" Longitude (West)=93°16'16.59"				SPT N60	MC (%)	COH (psf)	γ (pcf)	Soil	Other Tests Or Remarks		
DEPTH	Depth	Lithology	Classification	Drilling Operation	REC	RQD		Core Breaks	3	:		
1	Elev.	7	Classification	200	(%)	(%)	(ft)	Breaks	ĕ	or Member		
-	- - -				18	- - - - 9						
5	-			X	12	17						
+	-		Fine Sand, brown with IOS at 8', moist to saturated, medium dense to dense	11		+						
	-		dense to dense	X Z	13	_ 21 _						
10	-			PD	27	20						
	- - _ 14.0			PD	41	19						
15	880.3 - 16.5		Very Fine Sand, brown, saturated, dense		33	25						
+	- 877.8 -			PD PD	29	_ _ 24 _						
20 -	-			PD	15	24						
	- - -			PD	22	17						
25	-		Fine Sand, browns, saturated, medium dense to very dense	PD	67	22						
	-			PD	85	21						
30	-			PD	77	21						
+	-				72	20						
35	36.0			PD	73	22						
	858.3		Bottom of Hole - 36.0' Water measured at 8.5' with auger									
-			Field Creu	v Chief						Class: Edit: Date: 12/12/ DISTRICT-OVERHEAD SIGNS:		



State Project Bridge No. or Job Desc. Trunk Highway/Location								Boring I	Vo.		Ground Elevation		
8825-652 OH SIGN MN47-004 NB Various Metro				B various ivietro	1			T04			896.9 (DTM)		
ocation Ramsey County Coordinate System							e 21130				SHEET 1 of 1 Drilling 10/25/1		
				STA 523+81, 20'Lt	Han	nmer CN	VIE Auto	matic Ca	librated	Completed 10/25/1			
Latitude (North)=45°08'49.51" Longitude (West)=93°16'15.76"					-	SPT	MC	СОН	γ	Soil	Other Tests		
	Depth	logy			io	N 60	(%)	(psf)	(pcf)	S	Or Remarks		
DEPLIA	Elev.	Lithology	Cla	ssification	Drilling Operation	REC		ACL	Core Breaks	ock	Formation		
_	LIEV.				00	(%)	(%)	(ft)	Dicana	ľ	or Member		
+		· · ·			1		+						
İ			Fine Sand, brown, moist, loose			7	7						
1	4.0				17		+						
5	892.9		Loamy Fine Sand, dark-brown,	moist, very loose		3 -	12						
+	6.5				17		+						
1	890.4												
1			Fine Sand, browns, moist to sa	F	24	_ 11 _							
10+	-				31	17							
+	11.5			17		+							
İ	885.4					19	26						
1			Very Fine Sand, gray-brown, sa	aturated, medium dense			+						
15	-					25	24						
+	16.5				PD		+						
Ī	880.4				X	22	24						
+					PD		+						
20 +	-				X	29	24						
Ť					PD		†						
I					X	29	25						
+					PD		+						
25 +	-				X	15	24						
Ť			Fine Sand, browns, saturated,	medium dense to very dense	PD		Ť						
Į					\times	42	22						
+					PD		+						
30 +	-				X	56 ⁻	23						
1					PD]	1						
Į					X	50	22						
+		· · ·			PD]	+						
35+	36.0				X	66	23						
	860.9		Bottom of Hole - 36.0'		<u> </u>	•							
			Water measured at 11.2' with a	auger									
_				Field Crew	Chief						Class: Edit: Date: 12/12/ DISTRICT-OVERHEAD SIGNS:		



	tate Project Bridge No. or Job Desc. Trunk Highway/Location 825-652 OH SIGN I-94-688 Various Metro							Boring I	Vo.	Ground Elevation 935.3 (DTM)		
.ocatic	on Ram	nsey (County Coordinate System		Drill	Machine	9 21130	4 CME F	at Tire		SHEET 1 of 1	
				STA 2118+46, 31' Lt	Han	nmer CN	/IE Auto	matic Ca	librated		Drilling 11/14/17 Completed	
	Latit	ude (itude (West)=93°27'02.34"		SPT	МС	СОН	γ		Othor Tooto	
_	Depth	h B				Mea	(%)	(psf)	(pcf)	Soil	Or Remarks	
DEPTH		Lithology			Drilling Operation	REC	RQD	ACL	Core	z	Formation	
מ	Elev.	Ľi	Cla	ssification	Ope	(%)	(%)	(ft)	Core Breaks	Š	or Member	
+	-						+					
+	-				13	15	18					
1	-					13	10					
5	_	slightly plastic Silt Loam, browns, moist, stiff to very stiff			1,7	25	19					
+	-						+					
1	-					33	15					
+	- 40.0			1		+						
10	_ 10.0 925.3	×	slightly plastic Sandy Loam wit	h some Gravel, brown, moist,	\dashv \times	31	13					
1	11.5 923.8	0, 1	very stiff		-[7]		Ī				little rougher drilling 11.5'	
+	-	, ´o, o,				40	8			36	6'	
15 —	ò , '			13		†						
15	15+	,0,	Loamy Sand and Gravel, dark-	browns, moist, dense	X	35	6					
+	, '0,			1	4.0	+ ,						
+	- 19.0	0 0				48	4					
20	916.3	, '0',			7	20						
+	-	, 0,			F	20	+ -					
1	-	, °				24	2					
+	-	· ^			1		+					
25	_	ó , ´				29 ⁻	1					
	-	, ,			1		‡					
+	-	0 /0	Sand and Gravel, brown, damp	to dry, dense	X	31	1					
	-	, ,			<u> </u>		+					
30	-	, ,			X	30	1					
+	-	0,			15		+					
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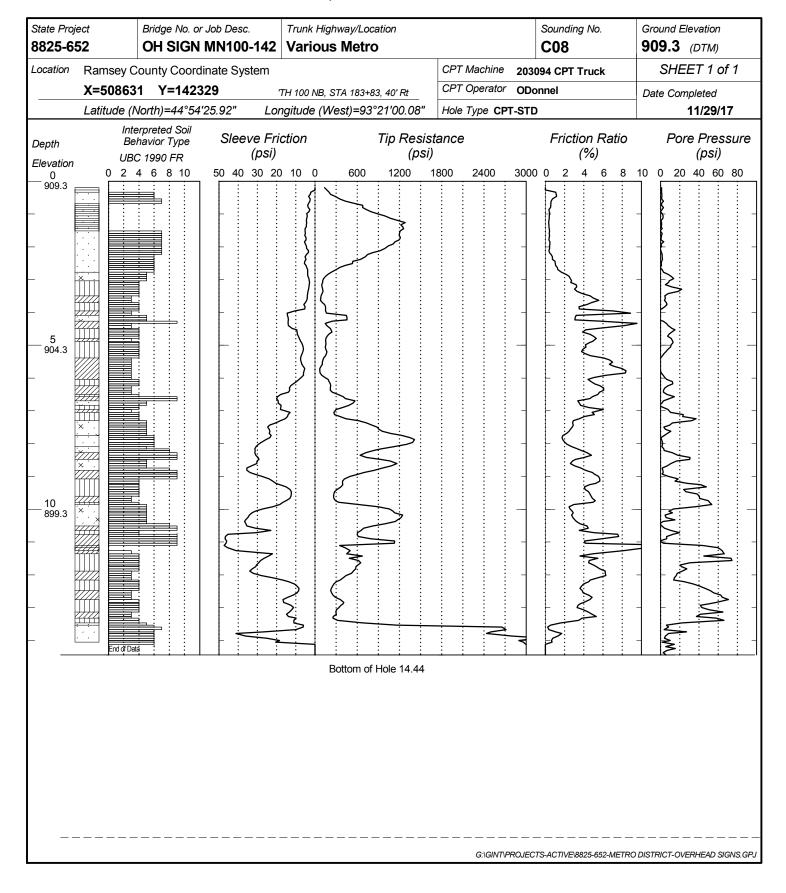
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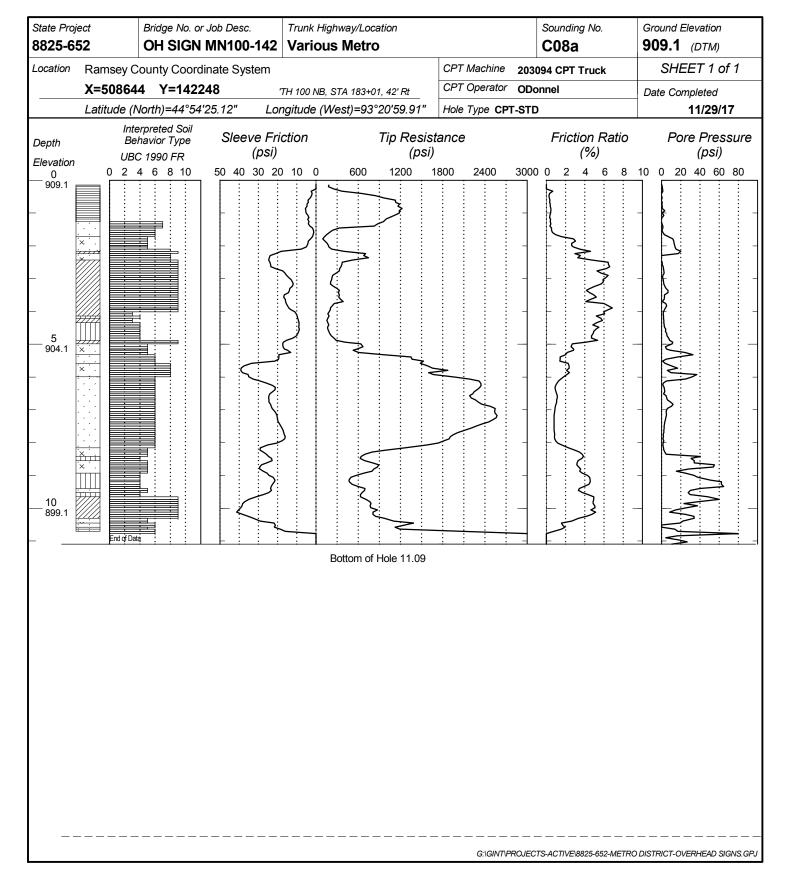


CONE PENETRATION TEST RESULTS



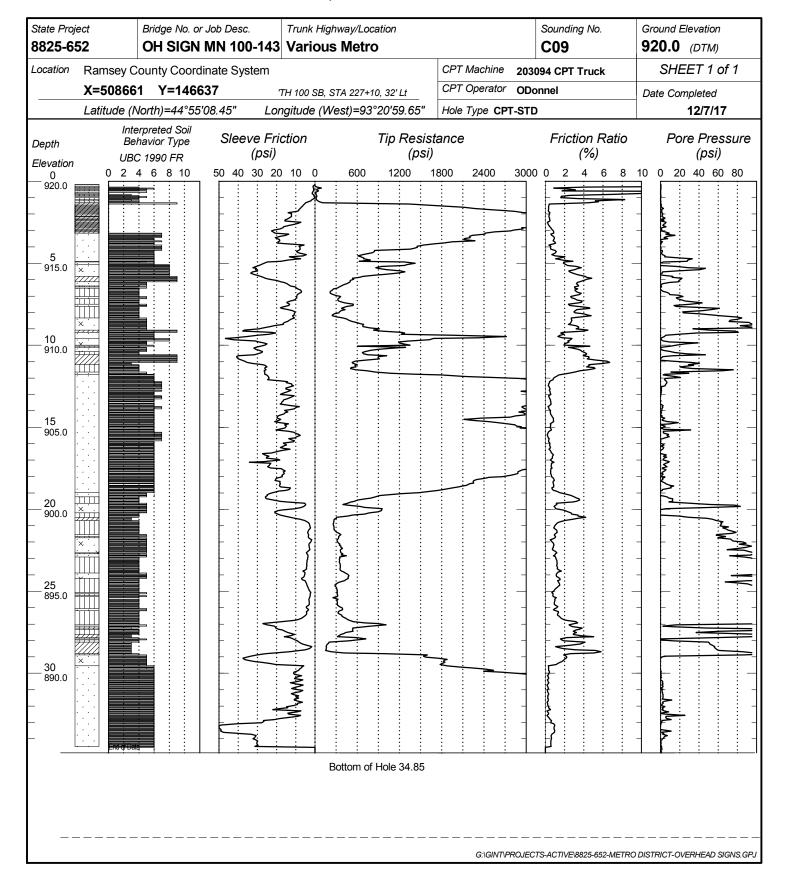


CONE PENETRATION TEST RESULTS





CONE PENETRATION TEST RESULTS





				Trunk Highway/Location Various Metro				Boring I	√o. (1969))	Ground Elevation 877.0 (from Plan)		
Locatio	ixan		County Coordinate System				e Soil S				SHEET 1 of 1 Drilling 4/9/69		
		Y=	01 0242	11, STA 135+25, 20' Lt	Hammer Rope & Cathead Completed						4/9/69		
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				Field Crew (Chief: C.	Thoms	en Soil C	lass:Tho	msen Ro	ck C	lass: Edit: Da	te: 12/13/17	
						G.\GII	VI IF NOUEC	10-ACTIVE	0020-002-IVI	LIKU	יטוווטוע -UVERNI	באוטוט.טר	



Minnesota Department of Transportation Geotechnical Section

BORING LOG DESCRIPTIVE TERMINOLOGY

USER NOTES, ABBREVIATIONS AND DEFINITIONS - Additional information available in Geotechnical Manual

This boring was made by ordinary and conventional methods and with care deemed adequate for the Department's design purposes. Since this boring was not taken to gather information relating to the construction of the project, the data noted in the field and recorded may not necessarily be the same as that which a contractor would desire. While the Department believes that the information as to the conditions and materials reported is accurate, it does not warrant that the information is necessarily complete. This information has been edited or abridged and may not reveal all the information which might be useful or of interest to the contractor. Consequently, the Department will make available at its offices, the field logs relating to this boring.

Since subsurface conditions outside each borehole are unknown, and soil, rock and water conditions cannot be relied upon to be consistent or uniform, no warrant is made that conditions adjacent to this boring will necessarily be the same as or similar to those shown on this log. Furthermore, the Department will not be responsible for any interpretations, assumptions, projections or interpolations made by contractors, or other users of this log.

Water levels recorded on this log should be used with discretion since the use of drilling fluids in borings may seriously distort the true field conditions. Also, water levels in cohesive soils often take extended periods of time to reach equilibrium and thus reflect their true field level. Water levels can be expected to vary both seasonally and yearly. The absence of notations on this log regarding water does not necessarily mean that this boring was dry or that the contractor will not encounter subsurface water during the course of construction.

WATER MEASUREMENT

AB After Bailing	
AC After Completion	
AF After Flushing	
wC with Casing	
wMWith Mud	
WSD While Sampling/Drilling	g
w /AUG With Hollow Stem A	- Auge

W /AUG WI	n Hollow Stem Auger
DRILLING OPERATION	ONS
AUG A	ugered
CD	ore Drilled
DBD	isturbed by Drilling
DBJ D	isturbed by Jetting
PDP	ug Drilled
ST Sp	olit Tube (SPT test)
TW TI	ninwall (3" Shelby Tube)
WS W	lash Sample
AB At	ter Bailing
NSR No	Sample Retrieved
WH W	eight of Hammer
WR W	
Mud Dr	illing Fluids in Sample
CS C	

MISCELLANEOUS

NA	Not Applicable
W	with
w/o	. with out
sat	saturated

SOIL CORE TESTS

SPT N ₆₀ ASTM D1586 Modified
Blows per foot with 140 lb. hammer and a
standard energy of 210 ft-lbs. This energy
represents 60% of the potential energy of the
system and is the average energy provided
by a Rope & Cathead system.
MC Moisture Content
COHCohesion (equivalent to 1/2
Unconfined Compression Strength)
Y Sample Unit Weight
LLLiquid Limit
PI Plasticity Index
Φ Angle of Internal Friction
REC Percent Core Recovered
RQD Rock Quality Description
(Percent of total core interval consisting of
unbroken pieces 4 inches or longer)
ACLAverage Core Length
(Average length of core that is greater than
4 inches long)
Core Breaks Number of natural core
breaks per 2 foot interval.

DISCONTINUITY SPACING

Fractures	Distance	Bedding
Very Close · · · · ·	<2 inches	Very Thir
Close ·····	2-12 inches	Thin
Mod. Close · · · · ·	12-36 inches	Medium
Wide · · · · · · ·	> 36 inches	Thick

RELATIVE DENSITY

Compactness - Granular Solls	DF F
very loose	. 0–4
loose	5-10
medium dense	. 11–24
dense	25-50
very dense	>50
Consistency - Cohesive Soils	RPF

Consistency - Cohesive Soils	<u>BPF</u>
very soft	0–1
soft	2–4
firm	5–8
stiff	9–15
very stiff	16-30
hard	31-60
very hard	>60

COLOR

blk Black	wht White
brn Brown	yel Ye l lov
orng Orange	lt Light
grn Green	dk dark
IOS Iron Oxide Stained	ar Grev

GRAIN SIZE /PLASTICITY

VF Very Fine	pl Plastic
F Fine	slpl Slightly Plastic
Cr Coarse	

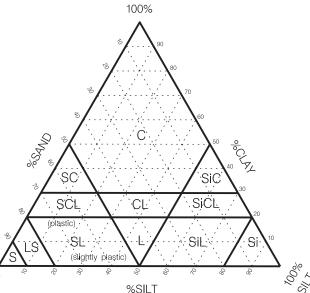
SOIL / ROCK TERMS

C Clay	Lmst Limestone
L Loam	Sst Sandstone
S Sand	Dolo Dolostone
Si Silt	wx weathered
G Gravel (No. 10 Sie	ve to 3 in.)
Bldr Boulder (over 3 i	nches dia.)
T till (unsorted, nonstr	ratified glacial deposits)

DRILLING SYMBOLS

Augered Plug Drilled (Rotary drilled with fluid) Split Tube Sample

Mn/DOT Triangular Textural Classification System Vane Shear Test CLAY Washed Sample (Collected during plug drilling)



Augered & Plug Drilled Jetted Jet Augered & Jetted

Core Drilled

Thin Wall Sample

Continuous Soil Sample

Index Sheet No.3.1 September 2016 <g:\geotech\public\forms\index31.dgn>



Minnesota Department of Transportation **Geotechnical Section**



Cone Penetration Test Index Sheet 1.0 (CPT 1.0)

USER NOTES, ABBREVIATIONS AND DEFINITIONS

This Index sheet accompanies Cone Penetration Test Data. Please refer to the Boring Log Descriptive Terminology Sheet for information relevant to conventional boring logs.

This Cone Penetration Test (CPT) Sounding follows ASTM D 5778 and was made by ordinary and conventional methods and with care deemed adequate for the Department's design purposes. Since this sounding was not taken to gather information relating to the construction of the project, the data noted in the field and recorded may not necessarily be the same as that which a contractor would desire. Department believes that the information as to the conditions and materials reported is accurate, it does not warrant that the information is necessarily This information has been edited or abridged and may not reveal all the information which might be useful or of interest to the contractor. Consequently, the Department will make available at its offices, the field logs relating to this sounding.

Since subsurface conditions outside each CPT Sounding are unknown, and soil, rock and water conditions cannot be relied upon to be consistent or uniform, no warrant is made that conditions adjacent to this sounding will necessarily be the same as or similar to those shown on this log. Furthermore, the Department will not be responsible for interpretations, assumptions, projections interpolations made by contractors, or other users of this log.

Water pressure measurements and subsequent interpreted water levels shown on this log should be used with discretion since they represent dynamic Dynamic Pore water measurements may deviate substantially from hydrostatic conditions, especially in cohesive soils. In cohesive soils, water pressures often take extended periods of time to reach equilibrium and thus reflect their true field level. Water levels can be expected to vary both seasonally and yearly. The absence of notations on this log regarding water does not necessarily mean that this boring was dry or that the contractor will not encounter subsurface water during the course of construction.

CPT Terminology

CPT Cone Penetration Test CPTU......Cone Penetration Test with Pore Pressure measurements SCPTU......Cone Penetration Test with Pore

Pressure and Seismic measurements

Piezocone...Common name for CPTU test

(Note: This test is <u>not</u> related to the Dynamic Cone Penetrometer DCP)

q_T TIP RESISTANCE

The resistance at the cone corrected for water pressure. Data is from cone with 60 degree apex angle and a 10 cm² end area.

fs SLEEVE FRICTION RESISTANCE

The resistance along the sleeve of the penetrometer.

FR Friction Ratio

Ratio of sleeve friction over corrected tip resistance.

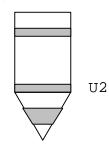
FR = fs/qt

V_s Shear Wave Velocity

A measure of the speed at which a siesmic wave travels through soil/rock.

PORE WATER MEASUREMENTS

Pore water measurements reported on CPT Log are representative of water pressures measured at the U2 location, just behind the cone tip, prior to the sleeve, as shown in the figure below. These measurements are considered to be dynamic water pressures due to the local disturbance caused by the cone tip. Dynamic water pressure decay and Static water pressure measurements are reported on a Pore Water Pressure Dissipation



SBT SOIL BEHAVIOR TYPE

Soil Classification methods for the Cone Penetration Test are based on correlation charts developed from observations of CPT data and conventional borings. Please note that these classification charts are meant to provide a guide to Soil Behavior Type and should not be used to infer a soil classification based on grain size distribution.

The numbers corresponding to different regions on the charts represent the following soil behavior types:

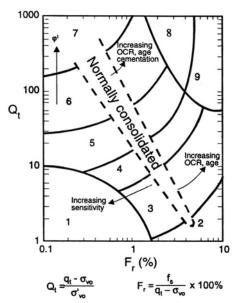
- 1. Sensitive, Fine Grained
- 2. Organic Soils Peats
- 3. Clays Clay to Silty Clay
- 4. Silt Mixtures Clayey Silt to Silty Clay
- 5. Sand Mixtures Silty Sand to Sandy Silt
- 6. Sands Clean Sand to Silty Sand
- Gravelly Sand to Sand
- Very Stiff Sand to Clayey Sand
- Very Stiff, Fine Grained

Note that engineering judgment, and comparison with conventional borings is especially important in the proper interpretation of CPT data in certain geomaterials.

The following charts are used to provide a Soil Behavior Type for the CPT Data.

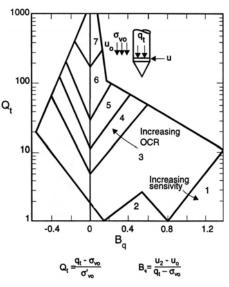
Robertson CPT 1990

Soil Behavior type based on friction ratio



Robertson CPTU 1990

Soil Behavior type based on pore pressure



where
QTnormalized cone resistance
Bq pore pressure ratio
FrNormalized friction ratio
σ_{VO} overburden pressure
$\sigma' vo$ effective over burden
pressure
u2 measured pore pressure
u ₀ equilibrium pore pressure

G:\GEOTECH\PUBLIC\FORMS\CPTINDEX.DOC January 30, 2002